

# Non-Circular Pressure Vessels

— Some guidance notes for designers

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## Introduction

The aim of this article is to set down, in readily available fashion, the fundamental theory needed for the design of some typical pressure vessels of non-circular cross-section.

Of these the most common are the rectangular section tanks. They are often used as bulk storage containers or as baths in the treatment of metals and fibres and surface coating processes etc. For this reason vessels of this type have been given special attention. Other shapes are also included by reference rather than by a worked example.

In explanation of the underlying theory a number of fully worked out examples are given showing the procedure which may be adopted when preparing the necessary calculation sheets for different types of vessels. Such calculation sheets may frequently be required for approval by Inspection and Insurance companies as well as Certification Authorities.

There are no national or international standards or codes of practice that will cover all of the types. Here ASME VIII, Div. 1, Appendix 13<sup>1</sup> probably offers the best guidance on a number of different designs. Unfortunately, in its present form, it is rather cumbersome and requires considerable time for proper understanding and assimilation. Another useful source of information on rectangular tanks can be found in the Theory and

Practical Design of Bunkers<sup>2</sup>. Rectangular section headers are also covered by the Swedish Pressure Vessels Code<sup>3</sup>, British Standard BS 1113: 1969<sup>4</sup> and the Italian Standard ANCC—VSR Collection Section VSR IS: 1976<sup>5</sup>. The last two references need to be viewed with considerable reservations as they appear to contain a number of discrepancies which are inconsistent with the fundamental theory.

Where there is no relevant code, the procedure outlined in this article follows the same logic, based on fundamental engineering theory as used in the codes and should therefore be equally acceptable. Such procedure should be regarded as evidence of good modern day general engineering practice in this field. It is hoped that it will promote a better understanding of the problems associated with such vessels which are often either ignored or not given the consideration and attention which they deserve. Such tanks can be quite complex in their detailed design and unawareness on the part of the designer and/or fabricator, to appreciate the various aspects can lead to costly ramifications later on. These tanks although they appear to be very simple indeed, can nonetheless cause considerable embarrassment if not assessed adequately at the outset.

Additional guidance is given on square/rectangular ducting. Normally such ducting is restricted to 20 psig (0.138 N/mm<sup>2</sup>). However, the

procedure outlined in this article has no limitation per se.

Comparison of the rectangular vessels with the equivalent size cylindrical (circular cross-section) vessels indicate that the former are rather inefficient. Cylindrical vessels will sustain considerably higher pressures, for the same wall thicknesses and size, see Fig. 20. However, practical considerations will often force the designer to select a rectangular shape as the best available option.

The fundamental theory is applicable to both external and internal pressure. Worked examples given in the text refer to internal pressure for the simple reason that, for the external pressure application, considerable gaps still exist in the knowledge of the allowable compressive stress levels which will not cause buckling or plastic collapse in rectangular and other non-circular tanks. In such instances it should be possible to use the design data contained in the British Standard BS 449: 1969<sup>6</sup> for checking the main stiffeners and beams.

## Fundamental theory for rectangular section pressure vessels

Figure 1 shows the basic geometry of the rectangular vessel with sharp corners and which is subjected to a uniform pressure of  $p$ .

- where  $L$  = the longer span
- $B$  = the shorter span
- $I_1$  = second moment of area of the beam  $BCB$  about its neutral axis
- $I_2$  = second moment of area of the beam  $BAB$  about its neutral axis

Due to symmetry about axes  $AA$  and  $CC$  it will be convenient to

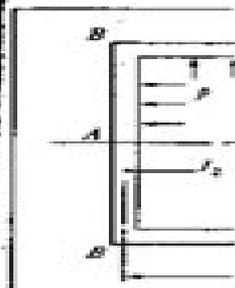


Fig. 1.

analyse one quadrant. The action of the  $p$  moments indicate

Clearly from (1) horizontal and vertical loads are acting on the quadrant

$$T_C = \frac{pB}{2}$$

which represents a load in member  $B$

$$T_A = \frac{pL}{2}$$

the tensile load in respectively. In  $p$  tensile forces the  $T_A$  and  $T_C$  are considered in comparison with such as  $L$  and  $B$ , i.e.

$$\left(\frac{L}{2} - \frac{T_A}{2}\right) \text{ effective}$$

In beams and rigid joints, as in the case, the strain energy direct and shear  $p$  in comparison with bending that only to be considered is statically indeterm

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